

ITEM 04502.802301 M - CONCRETE PLACEMENT FOR PARTIAL DEPTH PCC PAVEMENT REPAIRS

DESCRIPTION

This work consists of providing and placing concrete for partial depth portland cement concrete pavement repairs at various locations as specified in the plans. The Contractor shall be responsible for providing a performance concrete, as outlined in this specification, that allows the pavement repairs to be completed within the Maintenance and Protection of Traffic requirements of the contract plans.

MATERIALS

Materials shall conform to the following specifications:

Portland Cement Concrete - General	501
Portland Cement Concrete Pavement	502
Rapid Setting Concrete Repair Material	701-09
Pre-molded Resilient Joint Filler	705-07
Portland Cement Mortar Bonding Grout	705-22
Membrane Curing Compound	711-05
Non-Chloride Accelerator Admixture	Approved List
Sandblasting Sand	No. 2 Sandblasting Sand

The Contractor shall be solely responsible for designing a concrete mix for the pavement repairs. In addition, the contract documents will provide any additional job specific requirements that shall be adhered to along with this specification.

The concrete mix design has two development phases; a design batch phase and a field batch phase. The design batch phase represents the laboratory preparation of the material. The field batch phase is the use of the material on the project site for pavement repairs.

Design Batching.

Concrete mix designs to be used to make pavement repairs shall be submitted to the Regional Materials Engineer through the Engineer for acceptance. The following information will be used to ascertain the acceptability of concrete mix designs:

1. All materials used in the mix are from a NYSDOT approved source.
2. The design mix proportioning meets the design strength and material requirements as outlined below.
3. All necessary testing results have been provided.

Failure on the part of the Contractor to submit sufficient data to permit the Regional Materials Engineer to render an informed evaluation will result in the nonacceptance of the mix design. Submitted concrete design mixes and test results are for informational purposes only. The submission and acceptance of the concrete design mix does not relieve the Contractor of the responsibility of providing a performance concrete for pavement repairs or achieving the specified design criteria. A minimum of six (6) working days will be needed to evaluate the acceptability of a mix design. The Engineer will notify the Contractor when a mix design is accepted by the Regional Materials Engineer.

Field Batching.

The Contractor shall use the material on the project site for a typical full depth pavement repair. This trial run shall be used to by the Contractor to evaluate and correct material and repair operations. It is recommended that the Contractor use personnel for this trial that will work on the project.

No partial depth concrete pavement repairs may begin until the Contractor has an accepted concrete mix design, an accepted Quality Control Plan, and has demonstrated to the Engineer that repair operations follow the Quality Control Plan and result in effective repairs.

The Contractor shall only use an accepted concrete mix design to complete pavement repairs. If in the opinion of the Engineer, an accepted mix design continually fails to achieve the specified material and strength properties the mix

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design shall be disapproved. Any pavement repairs made with this mix may be rejected by the Engineer. The Contractor shall then return to the design batching phase to provide an acceptable mix design.

Concrete Strength Requirement.

The Contractor’s field batch shall achieve a minimum compressive strength of 17 MPa after the curing period chosen, that allows traffic to return to the repaired pavement within the Maintenance and Protection of Traffic requirements of the contract plans. In addition, the field batch shall have a minimum 28 day compressive strength of 21 MPa when Type III cement is used.

Concrete mix designs that have been used on previous projects and received approval from the Regional material Engineer, at the Engineer’s option may skip the fielding batching step. If this mix fails to meet design requirements the Contractor shall return to the design batching phase to provide an acceptable mix design.

The requirements of §501, Portland Cement Concrete - General, shall apply, except as modified in this specification.

§501-2.01 Composition of Mixtures.

The Contractor shall design a concrete mix that meets the following requirements:

- A. The design batch slump shall be 75 mm. The field batch slump shall be a maximum of 100 mm and a minimum of 40 mm.
- B. The design air content (entrapped plus entrained) shall be 7.5%. The field air content shall be a maximum of 9.0% and a minimum of 6.0%.
- C. The design batch shall achieve a minimum compressive strength of 17 MPa at the end of the select curing period and a minimum 28 day compressive strength of 30 Mpa.
- D. The field batch shall achieve a minimum compressive strength of 17 MPa at the end of the select curing period and a minimum 28 day compressive strength of 21Mpa when type III cement is used.
- E. The Contractor’s mix design shall determine the minimum curing period necessary for the mix or mixes to achieve the required compressive strength prior to opening the repaired pavement to traffic. (Note: that all strength testing shall be completed at the end of the curing period)
- F. The concrete mix design shall have a maximum water/cement ratio (weight) of 0.46.
- G. The coarse aggregate shall meet the requirements of table 501-2, type CA1.

Mix Design Properties	Minimum	Desired	Maximum
Slump	40 mm	-	100 mm
Air Content	6.0%	7.5%	9.0%
Design Batch, 28 day Compressive Strength	28 MPa	-	-
Design Batch, Selected Curing Period Compressive Strength	17 MPa	-	-
Field Batch, Selected Curing Period Compressive Strength	17 MPa	-	-
Field Batch, 28 day Compressive Strength (when type III used)	21 MPa	-	-
water/cement ratio			0.46

Rapid Setting Concrete Repair Material:

The Contractor may use a Rapid Setting Concrete Repair Material conforming to §701-09 of the Standard Specifications. This option shall be treated as the Contractors concrete mix design and all requirements of this specification shall apply. If the Contractor chooses this option, the Contractor will not be required to submit any information for the design batch phase other than the supplier and material to be used.

The Contractor shall include in the Quality Control Plan, the manufacturer’s instructions for preparing and

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placing the material. Testing results for air content and slump shall not be required. The concrete shall meet the strength requirement after curing the concrete according to the manufacturers recommendations. The Contractor shall reject any material not in conformance with the manufacturer's instructions and the quality control plan.

§501-2.02 Materials.

A. Cement: Type I, II, I/II or Type III Portland cement may be used. In Addition, if Type III cement is used the Contractor shall be required to perform 28 day compressive strengths on all field batches.

C. Admixtures: Use only neutralized vinsol resin based air entraining agents. Water reducers, if used, must be Type A (Normal). When needed only non-chloride accelerators shall be used. No more than one type of non-chloride accelerator may be used in any mix design.

§501-3.03 Concrete mixing, Transporting and Discharge.

Apply the following additions to sections 3.03C Central Mixed Concrete, 3.03D Transit Mixed Concrete.

Accelerator Solution added using Air Pressurized Tanks.

The truck mixer shall be equipped with air pressurized tanks meeting the following requirements:

- Sufficient capacity to supply the required solution quantity,
- Discharges the required solution quantity into the truck mixer drum in less than one minute,
- A tank output hose, made of clear plastic, leading into the truck mixer drum, and
- A properly working relief valve.

The non-chloride accelerator shall be added to the mix at the project location. The addition of the non-chloride accelerator shall be added only in the presence of the Engineer. For Central Mixed and Transit Mixed Concretes, after the addition of the non-chloride accelerator, the concrete shall be mixed for 30 mixing speed revolutions.

Accelerator Solution added by Contractor Method.

An alternate method for adding the non-chloride accelerator may be proposed in writing by the Contractor. The Regional Materials Engineer will review and accept or reject the Contractors proposed method.

§501-3.03 E, Truck Mixed Concrete will not be allowed.

Truck Mixed Concrete and Mobile Concrete Mixing Units shall not be allowed for this specification.

Small construction mixers or paddle mixers may be used to mix the concrete repair material. All mixers shall be subject to the Engineer's approval and be replaced when in the opinion of the Engineer that it is not working satisfactorily. The Contractor shall be responsible for ensuring that the mixer can mix the concrete repair material according to the manufactures instructions. Any repair material that does not achieve the desired results as determined by testing or the Engineer shall not be used to make repairs. The Contractor shall have a minimum of 60 minutes to prepare and place the repair material after mixing has been started.

Concrete Testing Requirements.

The Contractor shall be solely responsible for testing all concrete placed under this specification. All testing Equipment shall be properly calibrated as per the appropriate ASTM procedures. No vibrating will be allowed for any testing procedures. All testing shall be completed by an ACI Certified Testing Technician(s), using all ASTM

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Standard procedures for concrete testing as outlined below. All testing will be witnessed by the Engineer or his designee. Any testing not witnessed by the Engineer or his designee will result in the rejection of the concrete placement. The Contractor shall notify and provide the Engineer with any assistance needed so the testing procedures can be witnessed. If in the opinion of the Engineer, the concrete material being used is not achieving the design properties additional testing may be ordered.

The following ASTM Standard Testing Procedures shall be used to test the concrete material:

1. **C39 Compressive Strength of Molded Concrete Cylinders.**

The Contractor or the Testing Facility shall have the capability of testing the compressive strength of the cylinders. The compression testing machine may not be used on department projects without prior approval by the Regional Materials Group. The Engineer will verify that the testing machine has been calibrated within the last 12 months.

The testing of the concrete cylinders shall be done at the end of the curing period for the concrete mix being used. The curing time shall begin for a truck load of concrete after the application of the curing compound to the pavement placement in which the truck load was used. The cylinders shall have the curing compound applied at the same time as the last pavement repair made for that truck load. All concrete cylinders cast for testing shall be cured with the same treatments and at a location near to the concrete placement for the chosen time period.

The concrete cylinders shall be cast according to the testing frequency for the chosen mix, just prior to the completion of the pavement repairs or a cessation of work. The cast concrete cylinders represent the material used to make the pavement repairs. If the average compressive strength of the cylinders fails to meet the minimum design requirements then the Contractor shall reject all repairs made with that truck load of concrete.

The Engineer will provide form BR 300 Concrete Cylinder Report in sufficient quantity to the Contractor. The Contractor shall complete the form(s) and return them directly to the Engineer for evaluation prior to final acceptance of any pavement repairs that the cylinders represent.

The Contractor shall cast an additional set of cylinders per placement if requested by the Engineer. The State reserves the right to conduct any additional testing needed to verify the mix design requirements.

2. **C143 Slump of Portland Cement Concrete**

3. **C192 Making and Curing Concrete Test Specimens in the Laboratory**

4. **C231 Air Content of freshly Mixed Concrete by the Pressure Method.**

The Contractor shall calibrate the air pressure meter at least once per week. The Engineer may require the Contractor to calibrate more frequently if in the opinion of the Engineer the equipment is malfunctioning. The Engineer may witness the calibration at any time.

Testing Frequency.

The Contractor may do additional testing and cast more cylinders than required however all results shall be applied to the repair work that the testing or cylinders represent.

The Contractor shall complete all required testing for each truck load of concrete.

The Contractor may at his option cast a minimum of one set of cylinders for the days placement with the understanding that the cylinders represent that days placement. If the cylinders fail to meet the minimum strength requirements, the pavement repairs made for that day shall be rejected by the Contractor and replaced at no cost to the State.

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The Contractor shall provide the following information to the Engineer prior to beginning pavement repairs for a given work day:

1. The pavement repair location and area to be represented by each truck load of concrete.
2. The volume of concrete needed to complete the repairs.
3. The time expected for completing the placement of the truck load(s) of concrete.
4. The Contractor shall provide any changes to the above information if a work stoppage occurs during a placement.

The Contractor shall note that this information will be used to determine pavement repair acceptability and rejection. Accuracy of the information provided is the responsibility of the Contractor.

Minor changes or fluctuations in admixture dosage rates and the addition of water will be permitted. Any tested truck load of concrete that is modified for any reason shall be retested. The retesting shall consist of an air test, slump test and a casting of cylinders for compressive strength determination. No other changes to an accepted concrete mix design shall be allowed.

CONSTRUCTION REQUIREMENTS

The requirements of §502, Portland Cement Concrete Pavement as modified by EI 02-003 shall apply, except as modified in this specification.

§502-2 Materials and Equipment.

2.04 Equipment: C. Vibrators. Hand held internal type vibrators having a maximum diameter of 25 mm and are capable of operating through a frequency range of 6000 - 9000 vibrations per minute shall be used to thoroughly consolidate the concrete, leaving the concrete free from honeycomb, for the full width and depth of the concrete placement.

§502-3 Construction Requirements.

3.01 Weather limitations: Since the Contractor is responsible for the performance of the concrete material and it's placement, these weather requirements shall be altered to weather recommendations. Any pavement repairs damaged by the weather shall be rejected and corrected by the Contractor according to §502-3.14 Damaged or Defective Concrete.

3.05 fixed form paving: In addition, No walking through vibrated concrete will be permitted. Concrete pavement repairs shall be one continuous placement at the repair location and no stopping of concrete placement will be permitted unless ordered by the Engineer. If for any reason the work is suspended the Contractor shall treat the stoppage as the ending of the placement. A new placement shall begin when commencing work again. If a repair location can not be completed in one continuous placement, the concrete placed shall be removed and disposed of at the Contractors expense.

3.06 Jointn Construction: It shall be the responsibility of the Contractor to make the initial saw cut (stage 1) for the concrete pavement repairs. The Contractor shall make Stage I saw cuts in the pavement at the following locations or as directed by the Engineer:

To Reestablish a Longitudinal Joint - when the longitudinal joint is removed and replaced by the pavement repair.

For a New Transverse Joint Location - when the pavement removal abuts existing concrete and load transfer devices are installed as shown in the contract plans or as directed by the Engineer.

Reconstruction of a Transverse Joint - For a full slab placement where transverse joint supports are installed as shown in the contract plans or as directed by the Engineer.

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The Contractor may with the approval of the Engineer add a joint location to control cracking if the repair has an irregular shape. Any Contractor added joints shall be made at the Contractors expense.

The width and depth shall not be greater than the final stage II saw cut show in the contract documents. It is recommended that stage I saw cuts be completed as show on the concrete pavement standard sheets indicated in the contract plans.

An early entry or a green cut saw approved by the Director of Materials Bureau equipped with diamond saw blades, blade guards, depth of cut controls and guides that are capable of making straight cuts to the dimensions shown on the contract plans shall be used. The Contractor may also use the soft cutting option to make the stage I saw cuts. The cost for the stage I saw cut shall be included in this concrete placement item. Damage to the Pavement resulting from improper saw cutting, as determined by the Engineer, shall be repaired according to §502-3.14 Defective or Damaged Concrete.

3.11 Curing: replace class C and F with the Contractor's design mix. All concrete shall be cured for the curing period according to item E of the mix design properties for this specification.

3.15 Hardened Surface Test: This section will not apply to this specification.

3.16 Profilograph: This section will not apply to this specification.

3.18 Opening Pavement Repairs To Traffic:

in addition to, The Contractor shall be responsible for completing and opening the pavement repairs within the maintenance and protection of traffic guidelines specified in the contract plans.

Failure of the Contractor to meet the minimum strength requirement (17 MPa) and in addition when Type III cement is used the minimum 28 day strength requirement (21 Mpa) for the concrete pavement repairs shall not relieve the Contractor of his/her responsibility to re-open the roadway to traffic as outlined in the contract plans. Concrete pavement, represented by the cylinders cast, that has **not** achieved the minimum compressive strength and must be opened to traffic shall be deemed defective and replaced according to §502-3.14 Defective or Damaged Concrete.

Any concrete pavement repairs that crack, spalls or fails for any reason prior to the acceptance of the project shall be deemed defective and repaired according to §502-3.14 Damaged or Defective Concrete.

Quality Control Plan.

The Contractor shall prepare a Quality Control Plan for the project. The Quality Control Plan must outline a detailed description of the Contractors concrete quality control system. Guidelines for preparing a Quality Control Plan are provided in the special notes of the contract proposal. This system must provide reasonable assurance that the quality of the materials and the completed construction conforms to the contract specifications. The Contractor shall submit the Quality Control Plan to the Engineer in charge. A minimum of 10 days will be required by the Regional Materials Engineer for each review of the Quality Control Plan. The Contractor shall address or modify any testing procedures deemed necessary by the Regional Materials Engineer. The Engineer in charge will notify the Contractor in writing of the acceptance of the Quality Control Plan. The Contractor shall post a copy of the Quality Control Plan in the Engineer's field office for use by State personnel.

The Contractor shall appoint a Quality Control Plan Administrator to act as a point of contact for State personnel and to oversee the implementation of the Quality Control Plan. The Quality Control Plan Administrator shall be present for all placement and testing of concrete material. The Contractor shall notify the Regional materials Engineer a minimum of five days in advance for any permanent change in the Quality Control Plan Administrator. The Contractor may have an assistant Quality Control Plan Administrator to substitute for the Quality Control Plan Administrator for short term absences. The Engineer may order a work stoppage if in the opinion of the Engineer, the Contractor is not following the quality control plan and/or the an acceptable Quality Control Plan Administrator

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is absence during concrete testing and placement. The Contractor shall then appoint another Quality Control Plan Administrator for acceptance by the Engineer.

The Contractor shall follow the Project Plans, Specifications and the accepted Quality Control Plan. Any work not in conformance with the above requirement shall be rejected by the Contractors Quality Control Plan Administrator. Rejected work shall be corrected by the Contractor according to §502-3.14 Defective or Damaged Concrete at no cost to the State of New York. In addition the Engineer in Charge may reject any work not in conformance with the above requirements.

METHOD OF MEASUREMENT

The Engineer will calculate the volume of concrete in cubic meters placed from payment lines shown on the contract plans and the field measured longitudinal length of the pavement repaired.

BASIS OF PAYMENT

In the cubic meter bid price include the cost of all materials, equipment, and labor necessary to; design the concrete mixes, complete laboratory testing, prepare and implement a quality control plan, mix, place, test, finish, cure and make the stage I saw cut for the concrete pavement repairs.

This specification is
DisApproved